Inference for Unstructured Multisample Studies (Ch. 7.1 and 7.4)

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The one-way ANOVA model

Residuals and fitted values

Variance estimation

Standardized residuals

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The one-way ANOVA model

- Suppose we have:
 - Some response variable, Y
 - Some covariate factor, X, with levels i = 1, 2, ..., I and n_i observations at level i.
- The one-way ANOVA model, sometimes called the one-way normal model, is:

$$Y_{ij} = \mu_i + \varepsilon_{ij}$$

where:

- The ε_{ij} 's are iid $N(0, \sigma^2)$
- μ_i is the true mean response at level *i* of the factor.

•
$$j = 1, 2, \ldots, n_i$$
.

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$$Y_{ij} = \mu_i + \varepsilon_{ij}$$



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 Compressive strengths of 8 different formulas of concrete:



 But the order of the numbers given to the formulas is meaningless. It wouldn't make sense to do a simple linear regression of strength on formula. Inference for

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Instead of:

$$Y_i = \beta_0 + \beta_1 X_i + \varepsilon_i$$

with Y_i as strength and X_i as the formula index, we use:

$$Y_{ij} = \mu_i + \varepsilon_{ij}$$

where:

- *i* is the formula index, $i = 1, 2, \ldots, 8$
- ▶ *j* is the index of a specimen within the formula *i* group.

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Example: springs

Spring constants of three types of steel springs:

Empirical Spring Constants							
Type 1 Springs	Type 2 Springs	Type 3 Springs					
1.99, 2.06, 1.99 1.94, 2.05, 1.88 2.30	2.85, 2.74, 2.74 2.63, 2.74, 2.80	2.10, 2.01, 1.93 2.02, 2.10, 2.05					



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Example: springs

- Doesn't make sense to regress exponential spring constant on spring type.
- Instead, we apply:

$$Y_{ij} = \mu_i + \varepsilon_{ij}$$

where:

- Y_{ij} is the exponential spring constant of spring type i spring number j.
- μ_i is the true mean exponential spring constant of type
 i.
- *i* is the formula index, $i = 1, 2, \ldots, 8$
- ▶ *j* is the index of a specimen within the formula *i* group.

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Fitted values

- Similarly to before, ŷ_{ij} is the fitted value corresponding to y_{ij}. It represents an estimate of the true mean response at factor level *i* and sample unit *j*.
- We treat all sample units equally, letting;

$$\widehat{y}_{ij} = \overline{y}_{i.} = rac{1}{n_i} \sum_{j=1}^{n_i} y_{ij}$$

the average of all the responses at factor level i.

• We get $\hat{y}_{ij} = \overline{y}_{i}$ by minimizing the loss function:

$$S(\mu_1,\mu_2,\ldots,\mu_I)=\sum_{ij}(y_{ij}-\mu_i)^2$$

over all the choices of $\mu_1, \mu_2, \ldots, \mu_l$, selecting \overline{y}_{i} to estimate μ_i .

The residuals e_{ij} are then:

$$e_{ij} = y_{ij} - \overline{y}_{i.}$$

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Example Computations of Residuals for the Concrete Strength Study

Specimen	<i>i</i> , Concrete Formula	y _{ij} , Compressive Strength (psi)	$ \hat{y}_{ij} = \bar{y}_i, $ Fitted Value	$e_{ij}^{},$ Residual
1	1	5,800	5,635.3	164.7
2	1	4,598	5,635.3	-1,037.3
3	1	6,508	5,635.3	872.7
4	2	5,659	5,753.3	-94.3
5	2	6,225	5,753.3	471.7
:	:	:	÷	:
22	8	2,051	2,390.7	-339.7
23	8	2,631	2,390.7	240.3
24	8	2,490	2,390.7	99.3

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We can compute a sample variance for each factor level:

$$s_i^2 = rac{1}{n_i - 1} \sum_j (y_{ij} - \overline{y}_{ij})^2$$

And we can compute a **pooled sample variance**:

$$s_P^2 = rac{(n_1-1)s_1^2 + (n_2-1)s_2^2 + \dots + (n_l-1)s_l^2}{(n_1-1) + (n_2-1) + \dots + (n_l-1)}$$

• The pooled sample standard deviation is just $s_P = \sqrt{s_P^2}$

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If $n = \sum_{i} n_i$, then:	Multisample Studies (Ch. 7.1 and 7.4)
$s_P^2 = \frac{(n_1 - 1)s_1^2 + (n_2 - 1)s_2^2 + \dots + (n_l - 1)s_l^2}{(n_l - 1)s_l + (n_l - 1)s_l + (n_l - 1)s_l}$	Will Landau
$(n_1 - 1) + (n_2 - 1) + \cdots + (n_l - 1)$	The one-way
$(n_1-1)\left(\frac{1}{n_l-1}\right)\sum_i(y_{1j}-\overline{y}_1)^2+\cdots+(n_l-1)\left(\frac{1}{n_l-1}\right)\sum_i(y_{lj}-\overline{y}_l)^2$	ANOVA model
$= (n_1 + 1)(n_2 + 1)(n_1 + 1)(n_2 + 1)(n_2 + 1)(n_1 + 1)(n_2 +$	Residuals and
n-1	fitted values
$=\frac{1}{n-I}\sum_{ij}(y_{ij}-\overline{y}_i)^2$	Variance estimation
$=\frac{1}{n-1}\sum_{j}e_{ij}^{2}$	Standardized residuals
n , jj	Inference

As it turns out,

$$E(s_P^2) = \sigma^2$$
$$\frac{n-l}{\sigma^2} s_P^2 \sim \chi_{n-l}^2$$

• A $1 - \alpha$ confidence interval for σ^2 is of the form:

$$\left(\frac{n-l}{\chi^2_{n-l,\ 1-\alpha/2}}s_P^2,\ \frac{n-l}{\chi^2_{n-l,\ \alpha/2}}s_P^2\right)$$

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-			-
<i>i</i> , Concrete Formula	n _i , Sample Size	ÿ _i , Sample Mean (psi)	<i>s_i</i> , Sample Standard Deviation (psi)
1	$n_1 = 3$	$\bar{y}_1 = 5,635.3$	$s_1 = 965.6$
2	$n_2 = 3$	$\bar{y}_2 = 5,753.3$	$s_2 = 432.3$
3	$n_{3} = 3$	$\bar{y}_3 = 4,527.3$	$s_3 = 509.9$
4	$n_{4} = 3$	$\bar{y}_4 = 3,442.3$	$s_4 = 356.4$
5	$n_{5} = 3$	$\bar{y}_5 = 2,923.7$	$s_5 = 852.9$
6	$n_{6} = 3$	$\bar{y}_6 = 3,324.7$	$s_6 = 353.5$
7	$n_7 = 3$	$\bar{y}_7 = 1,551.3$	$s_7 = 505.5$
8	$n_8 = 3$	$\bar{y}_8 = 2,390.7$	$s_8 = 302.5$

Summary Statistics for the Concrete Strength Study

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$$s_P^2 = \frac{(3-1)(965.6)^2 + (3-1)(432.3)^2 + \dots + (3-1)(302.5)^2}{(3-1) + \dots + (3-1)}$$

= $2\frac{965.6^2 + 432.3^2 + \dots + 302.5^2}{16}$
= 338213 psi^2
 $s_P = \sqrt{338213} = 581.6\text{psi}$

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•
$$n = 24$$
, $l = 8$, $n - l = 16$.
• $\chi^2_{16, 0.95} = 26.296$, $\chi^2_{16, 0.05} = 7.962$
• Hence, a 90% 2-sided confidence interval for σ^2 is:

$$\left(\frac{16\cdot 581.6^2}{26.296}, \ \frac{16\cdot 581.6^2}{7.962}\right) = (205816, \ 679745.9)$$

and you can make a 90% confidence interval for σ by transforming the endpoints of the confidence interval for σ^2 :

$$(\sqrt{205816}, \sqrt{679745.9}) = (453.7, 824.5)$$

 We're 90% confident that the true overall standard deviation of compressive strength of the concrete within factor levels is between 453.7 psi and 824.5 psi.

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- Just as before, even though ε_{ij} ~ iid N(0, σ²), the e_{ij}'s don't have constant variance.
- The standardized residuals for the one-way ANOVA model are of the form:

$$e_{ij}^{*}=rac{e_{ij}}{s_{P}\sqrt{rac{n_{i}-1}{n_{i}}}}$$

which are about N(0,1) on average.

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- 1. $H_0: \mu_1 = \mu_2 = \cdots = \mu_I$, H_a : not all the μ_i 's are equal.
- 2. α is some sensible value.
- 3. The test statistic is:

$$K = \frac{MSR}{MSE} = \frac{SSR/(I-1)}{SSE/(n-I)}$$

Here,

- *n* is the number of observations.
- I is the number of levels of the covariate.

$$\blacktriangleright SSR = \sum_{ij} (\widehat{y}_{ij} - \overline{y}_{..})^2 = \sum_{ij} (\overline{y}_{i.} - \overline{y}_{..})^2$$

•
$$SSE = \sum_{ij} (y_{ij} - \widehat{y}_{ij})^2 = \sum_{ij} (y_{ij} - \overline{y}_{i.})^2$$

►
$$SST = \sum_{ij} (y_{ij} - \overline{y}_{..})^2$$

► $\overline{y}_{..} = \frac{1}{n} \sum_{ii} y_{ij}$

- Assume H₀ is true, the model is valid, and the ε_{ij}'s are iid N(0, σ²)
- Then, $K \sim F_{I-1, n-I}$.
- Reject H_0 if $K > F_{I-1, n-I, 1-\alpha}$

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4. The moment of truth: construct the ANOVA table:

Source	SS	df	MS	F
Covariate	SSR	I - 1	SSR/(I-1)	MSR/MSE
Error	SSE	n — I	SSE/(n-I)	

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1.
$$H_0: \mu_1 = \mu_2 = \cdots = \mu_8$$
, H_a : not all the μ_i 's are equal

- **2**. $\alpha = 0.05$
- 3. The test statistic is:

$$K = \frac{MSR}{MSE} = \frac{SSR/(I-1)}{SSE/(n-I)} = \frac{SSR/7}{SSE/16}$$

- Assume H₀ is true, the model is valid, and the ε_{ij}'s are iid N(0, σ²)
- Then, $K \sim F_{I-1, n-I}$.
- ▶ Reject H_0 if $K > F_{I-1, n-I, 1-\alpha} = F_{7,16,0.95} = 2.66$

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 The moment of truth: we start by calculating SST, s²_P, and SSE:

$$(5,800 - 3,693.6)^{2} + (4,598 - 3,693.6)^{2} + (6,508 - 3,693.6)^{2} + \dots + (2,631 - 3,693.6)^{2} + (2,490 - 3,693.6)^{2} = 52,772,190 \text{ (psi)}^{2}$$

$$s_{\rm P}^2 = 338,213.1 \ ({\rm psi})^2 \ {\rm and} \ n-r = 16, {\rm so}$$

$$SSE = (n - r)s_{\rm P}^2 = 5,411,410 \, (\rm psi)^2$$

Lastly, we calculate SSR:

$$\sum_{i=1}^{r} n_i (\bar{y}_i - \bar{y})^2 = 47,360,780$$

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ANOVA Table (for testing $H_0: \mu_1 = \mu_2 = \cdots = \mu_8$)							
Source	SS	df	MS	F			
Treatments	47,360,780	7	6,765,826	20.0			
Error	5,411,410	16	338,213				
Total	52,772,190	23					

- 5. With K = 20.0 > 2.66, we reject H_0 and conclude H_a .
- 6. There is enough evidence to conclude that the compressive strength of the concrete varies with formula.

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- The following data are taken from the paper Zero- Force Travel-Time Parameters for Ultrasonic Head-Waves in Railroad Rail by Bray and Leon- Salamanca (Materials Evaluation, 1985).
- Given are measurements in nanoseconds of the travel time (in excess of 36.1 µs) of a certain type of mechanical wave induced by mechanical stress in railroad rails.

Rail	Travel Time (nanoseconds above 36.1 μ s)
1	55, 53, 54
2	26, 37, 32
3	78,91,85
4	92, 100, 96
5	49, 51, 50
6	80, 85, 83

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We apply the model:

$$Y_{ij} = \mu_i + \varepsilon_{ij}$$

where:

- Y_{ij} is the observed travel time (ns) of the wave in excess of 26.1 μs for Rail i wave j.
- μ_i is the true mean travel time (ns) in excess of 26.1 μs of waves through Rail i.

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1.
$$H_0: \mu_1 = \mu_2 = \cdots = \mu_6$$
, H_a : not all the μ_i 's are equal.

- **2**. $\alpha = 0.05$
- 3. The test statistic is:

$$K = \frac{MSR}{MSE} = \frac{SSR/(I-1)}{SSE/(n-I)} = \frac{SSR/(6-1)}{SSE/(18-6)} = \frac{SSR/5}{SSE/12}$$

- Assume H₀ is true, the model is valid, and the ε_{ij}'s are iid N(0, σ²)
- Then, $K \sim F_{I-1, n-I}$.

• Reject
$$H_0$$
 if $K > F_{I-1, n-I, 1-\alpha} = F_{5,12,0.95} = 3.11$

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4. The moment of truth: load the data into JMP and fit travel time on rail, and *make sure the rail variable is a factor*.

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Analysis of Variance								
Sum of Source DE Squares Mean Square E Batio								
Model	5	9310.5000	1862.10	115.1814				
Error C. Total	12 17	194.0000 9504.5000	16.17	Prob > F <.0001*				

- 5. With K = 115.18 > 3.11, we reject H_0 and conclude H_a .
- 6. There is enough evidence to conclude that the true mean excess travel time of waves along the rails depends on the rail.

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